GENERAL NOTES

1.00 GENERAL NOTES

WIND PROVISIONS OF ASSUMPTIONS. N SHALL OF ASCE CONFORM TO E 7-02. SEE W WIND 3HT 2004 LOAD INTERNATIONAL BUILDING CODE AND THE DIAGRAM FOR WIND SPEED AND DESIGN

1.02 DESIGN GRAVITY LOADS ARE AS FOLLOWS:

- SUPERIMPOSED DEAD LOADS:

 1. MECHANICAL, ELECTRICAL, PLUMBING:
 2. CEILING: 5 PSF
 3. PARTITIONS: 20 PSF 5
- SUPERIMPOSED LIVE LOADS: (MAY BE REDUCED 1. ROOFS: 20 PSF 2. MEZZANINE (STORAGE): 150 PSF PER CODE)
- C. SEISMIC ASSUMPTIONS: SEISMIC DESIGN IS IN AC 1. SEISMIC GROUP: I 2. PERFORMANCE LEVEL: LS, LIFE SAFETY 3. PERMORMANCE OBJECTIVE: 1A 4. SITE CLASSIFICATION: D Ss = 0.099g Sds = 0.1056g S1 = 0.056g Sd1 = 0.0896g
- 1.03 DRAWINGS SHOW TYPICAL AND CERTAIN SPECIFIC CONDITIONS ONLY. FOR DETAILS NOT SPECIFICALLY SHOWN, PROVIDE DETAILS SIMILAR TO THOSE SHOWN.
- VERIFY ALL EXISTING CONDITIONS, DIMENSIONS AND ELEVATIONS BEFORE STARTING WORK. NOTIFY THE CONTRACTING OFFICER OF ANY DISCREPANCY.
- 1.05 THE DESIGN, ADEQUACY, AND SAFETY OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC., ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- 1.06 COORDINATE STRUCTURAL CONTRACT DOCUMENTS WITH ARCHITECTURAL, MECHANICAL, ELECTRICA PLUMBING AND CIVIL. NOTIFY CONTRACTING OFFICER OF ANY CONFLICT AND/OR OMISSION. CONTRACTOR SHALL MAKE NO DEVIATION FROM DESIGN DRAWINGS WITHOUT WRITTEN APPROVAL OF THE ARCHITECT. FOR ADDITIONAL OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS, SEE ARCHITECTURAL, MECHANICAL, AND PLUMBING DRAWINGS. **ELECTRICAL**,
- 1.07 REVIEW OF SUBMITTALS AND/OR SHOP DRAWINGS BY THE CONTRACTING OFFICER DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO REVIEW AND CHECK SHOP DRAWINGS BEFORE SUBMITTAL TO THE CONTRACTING OFFICER. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS. CONTRACTOR IS ALSO RESPONSIBLE FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.

2.00 FOUNDATIONS AND SLAB-ON-GRADE

2.01 SHALLOW FOUNDATIONS HAVE BEEN DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 1500 PSF. AT THIS TIME NO GEOTECHINCAL REPORT HAS BEEN SUBMITTED. AN ALLOWABLE SOIL BEARING PRESSURE HAS BEEN CONSERVATIVELY ESTIMATED BASED ON SIMILAR PROJECTS OF THIS SIZE AND LOCATED IN THE SAME GENERAL AREA OF CONSTRUCTION. A QUALIFIED GEOTECHNICAL ENGINEER SHALL VERIFY CONDITION AND/OR ADEQUACY OF ALL SUBGRADES, FILLS AND BACKFILLS BEFORE PLACEMENT OF FOUNDATIONS, FOOTINGS, SLABS, WALLS, FILLS, BACKFILLS, ETC. SHOULD THE CONTRACTOR FIND UNDESIRABLE SOILS, HE SHALL STOP WORK AND IMMEDIATELY CONTACT THE CONTRACTING OFFICER. ALL FOOTINGS SHALL REST EITHER ON UNDISTURBED SOIL OR A MANUALLY OPERATED VIBRATORY SLED OR TAMPER SHOULD BE USED TO DENSIFY ANY SOILS IN THE BOTTOM OF THE FOOTING TRENCHES LOOSENED DURING THE EXCAVATION OPERATION.

2.02 SIDES OF FOUNDATIONS SHALL BE FORMED.

- 2.03 CONTRACTOR IS RESPONSIBLE FOR ADEQUATELY PROTECTING ALL EXCAVATION SLOPES.
- 2.04 DEWATER TO AT LEAST TWO FEET BELOW BOTTOM OF LOWEST FOUNDATION IF GROUNDWATER IS ENCOUNTERED.

2.05 SLAB-ON-GRADE REQUIREMENTS:

- THE SLAB-ON-GRADE SHALL BE A MINIMUM OF 6 INCHES THICK, PLACED ON COMPACTED SUBGRADE, AND REINFORCED WITH $6\times6-W7.5\times W7.5$ WWF IN FLAT SHEETS (ROLLS ARE NOT PERMITTED). PROVIDE POSITIVE SUPPORT 3" CLEAR FROM BOTTOM OF SLAB. LAP MESH 12".
- PLACE CONTROL JOINTS AS SHOWN AND AT INTERMEDIATE LINES SUCH THAT AREA OF EACH PANEL DOES NOT EXCEED 400 SQUARE FEET, U.N.O.
- IF REQUIRED, LOCATE CONSTRUCTION JOINTS AT CONTROL JOINTS. SAWCUT CONTROL AS SOON AFTER POURING AS POSSIBLE, WHEN CONCRETE WILL NOT RAVEL; 24 HRS. CURE CONCRETE IN ACCORDANCE WITH ACI 301. BEGIN CURING IMMEDIATELY AFTER POURING TO LIMIT CRACKING PRIOR TO SAWCUTTING CONTROL JOINTS.
- TYPE D GRANULAR FILL CAPILLARY BARRIER UNDER SLABS-ON-GRADE: 6" MINIMUM THICKNESS OF A COMPACTIBLE, EASY-TO TRIM, GRANULAR FILL THAT WILL REMAIN AND SUPPORT CONSTRUCTION TRAFFIC. STABLE
- CLASS B REINFORCED MULTI-PLY VAPOR RETARDER SHOULD BE PLACED UNDER THE GRANULAR FILL CAPILLARY BARRIER. IF THE VAPOR BARRIER/RETARDER IS TO BE PLACED OVER A ROUGH GRANULAR FILL, A THIN LAYER OF APPROXIMATELY 1/2 IN. OF FINE—GRADED MATERIAL SHOULD BE ROLLED OR COMPACTED OVER THE FILL PRIOR TO INSTALLATION OF THE VAPOR BARRIER/RETARDER TO REDUCETHE POSSIBILITY OF PUNCTURE. VAPOR BARRIERS/RETARDERS SHOULD BE OVERLAPPED 6 IN. AT THE JOINTS AND CAREFULLY FITTED AROUND SERVICE OPENINGS.

3.00 REINFORCED CONCRETE

3.01 ALL CONCRETE WORK SHALL CONFORM TO ACI 301, SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS. DESIGN IS BASED ON ACI 318, BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE. DETAIL CONCRETE REINFORCEMENT AND ACCESSORIES IN ACCORDANCE WITH ACI 315, DETAILING MANUAL. DETAIL ALL CONCRETE WALLS AND BEAMS ON THE SHOP DRAWINGS IN ELEVATION UNLESS SPECIFICALLY APPROVED OTHERWISE. SUBMIT SHOP DRAWINGS FOR APPROVAL, SHOWING ALL FABRICATION DIMENSIONS AND LOCATIONS FOR PLACING REINFORCING STEEL AND ACCESSORIES. DO NOT BEGIN FABRICATION UNTIL SHOP DRAWINGS ARE COMPLETED AND REVIEWED.

- 3.02 UNLESS NOTED OTHERWING MINIMUM 28 ISE, ALL CONCRETE SHALL BE NORMAL WEIGHT DAY COMPRESSIVE STRENGTHS: AND HAVE THE
- FOUNDATIONS
 SLAB-ON-GRADE MONOLITHIC 3000 PSI
 CONCRETE SHALL HAVE 2% 6% ENTRAINED AIR.
- 3.03 THE PROPOSED MATERIALS AND MIX DESIGN SHALL BE FULLY DOCUMENTED AND REVIEWED BY THE CONTRACTING OFFICER AND APPROVED TESTING LABORATORY. OBTAINING THE REQUIRED DESIGN STRENGTH IS THE CONTRACTOR'S RESPONSIBILITY.
- 3.04 USE OF CALCIUM CHLORIDE, CHLORIDE IONS, OR OTHER SALTS IN CONCRETE IS NOT PERMITTED.

3.05 CHAMFER OR ROUND ALL EXPOSED CORNERS A MINIMUM OF

- 3.06 REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60 UNLESS NOTED OTHERWISE.
- 3.07 TIE ALL REINFORCING STEEL AND EMBEDMENTS SECURELY IN PLACE PRIOR TO PLACING CONCRETE. PROVIDE SUFFICIENT SUPPORTS TO MAINTAIN THE POSITION OF REINFORCEMENT WITHIN SPECIFIED TOLERANCE DURING ALL CONSTRUCTION ACTIVITIES. "STICKING" DOWELS IN WET CONCRETE IS NOT PERMITTED. OTN O
- 3.08 PROVIDE CONTINUOUS REINFORCEMENT WHEREVER POSSIBLE; SPLICE ONLY AS SHOWN OR APPROVED; STAGGER SPLICE WHERE POSSIBLE; USE FULL TENSION SPLICE (CLASS "B") UN NOTED OTHERWISE. DOWELS SHALL MATCH THE SIZE AND SPACING OF THE SPECIFIED REINFORCEMENT AND SHALL BE LAPPED WITH FULL TENSION SPLICES (CLASS "B") UNLESS NOTED OTHERWISE. TERMINATE BARS WITH STANDARD HOOKS. UNLESS
- 3.09 REINFORCING STEEL SHALL HAVE THE FOLLOWING CONCRETE COVER UNLESS NOTED OTHERWISE:
- A. FORMED CONCRETE EXPOSED TO THE EARTH OR WEATHER:

 1. #6 THROUGH #18 BARS: 2"

 2. #5 BARS AND SMALLER: 1-1/2"
- #18 BARS: 2" D SMALLER: 1-1/2"
- 3.10 DO NOT WELD OR TACK WELD REINFORCING STEEL
- 3.11 ALL REINFORCING STEEL PLACEMENTS SHALL BE REVIEWED BY A REGISTERED STRUCTURAL ENGINEER, OR BY A REPRESENTATIVE RESPONSIBLE TO HIM. (RE: ACI 318 PAR. 1.3.1)
- 3.12 FOR CONCRETE PADS SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS
- 4.00 STRUCTURAL STEEL, STEEL DECK
- 4.01 STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED ACCO "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, ASD, LATEST EDITION. ACCORDING TO AISC
- 4.02 SUBMIT SHOP DRAWINGS PREPARED IN ACCORDANCE WITH AISC MANUAL "DETAILING FOR STEEL CONSTRUCTION", LATEST EDITION. STEEL FABRICATOR SHALL SUPPLY ANCHOR BOLT LOCATION DRAWINGS. DO NOT BEGIN FABRICATION UNTIL SHOP DRAWINGS ARE COMPLETED AND REVIEWED.
- 4.03 STRUCTURAL STEEL WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992. STRUCTURAL STEEL SHAPES, PLATES, ANGLES, AND CHANNELS SHALL CONFORM TO ASTM A36 UNLESS NOTED OTHERWISE. STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B, FY = 46 KSI, UNLESS NOTED OTHERWISE. STEEL PIPE SHALL CONFORM TO ASTM A501 OR ASTM A53, TYPE I OR S, GRADE B. ANCHOR BOLTS SHALL CONFORM TO ASTM A36 UNLESS NOTED OTHERWISE.
- 4.04 BOLTS SHALL CONFORM TO ASTM A325, 3/4-INCH DIAMETER MINIMUM, UNLESS NOTED OTHERWISE. BOLTS IN BEARING CONNECTIONS SHALL BE DESIGNATED TYPE N, TENSIONED, SNUG-TIGHT AS DEFINED BY AISC. BOLTS IN MOMENT CONNECTIONS AND IN TRUSSES SHALL DESIGNATED SLIP-CRITICAL (SC). FULLY TENSION SLIP-CRITICAL CONNECTIONS WITH DIRECT TENSION INDICATOR WASHERS INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTION.
- 4.05 HEADED STUD SHEAR CONNECTORS (INDICATED AS "HS" ON PLANS): ASTM A 108, GRADES 1010 THROUGH 1020, HEADED—STUD TYPE, COLD—FINISHED CARBON STEEL; AWS D1.1, TYPE B. USE AUTOMATIC END WELDING OF HEADED—STUD SHEAR CONNECTORS ACCORDING TO AWS D1.1 AND MANUFACTURER'S WRITTEN INSTRUCTIONS.
- 4.06 USE PRE-QUALIFIED WELDED JOINTS AS PER AISC, AND AWS D1.1 "STRUCTURAL WELDING CODE." USE ONLY CERTIFIED WELDERS; ALL ELECTRODES SHALL CONFORM TO AWS A5 GRADE E70XX. BARE ELECTRODE AND GRANULAR FLUX SHALL CONFORM TO AWS A5, F70 AWS FLUX CLASSIFICATION. MINIMUM WELD SIZE TO BE 3/16" FILLET WELD, U.N.O.
- 4.07 CUTS, BOLTS, COPING, ETC THE SHOP DRAWINGS AND STEEL MEMBERS IN THE F ETC. REQUIRED FOR WORK OR OTHER TRADES SHALL BE SHOWN ON NADE IN THE SHOP. CUTS OR BURNING HOLES IN STRUCTURAL FIELD WILL NOT BE PERMITTED.
- 4.08 SHOP CONNECTIONS NOT SPECIFICALLY DETAILED ON THE DRAWINGS MAY BE WELDED OR BOLTED. FIELD CONNECTIONS NOT SPECIFICALLY DETAILED ON THE DRAWINGS SHALL BE BOLTED, WHERE POSSIBLE.
- 4.09 WHEN SPECIFICALLY NOT CONNECTIONS: DETAILED ON THE DESIGN DRAWINGS PROVIDE THE FOLLOWING BEAM
- USE TWO—SIDE ANGLE CONNECTIONS PER AISC SPECIFICATIONS, LATEST EDITION. WHERE BEAM REACTIONS ARE SHOWN, CONNECTIONS SHALL DEVELOP THE REACTION GIVEN. WHEN BEAM REACTIONS ARE NOT SHOWN, CONNECTIONS SHALL BE DESIGNED TO SUPPORT ONE HALF THE TOTAL UNIFORM LOAD CAPACITIES SHOWN IN THE ALLOWABLE UNIFORM LOAD TABLES, PARTS 2 OF THE AISC MANUAL, FOR THE GIVEN BEAM, SPAN, AND GRADE OF STEEL SPECIFIED.

 WHERE REACTIONS ARE SUBJECT TO ECCENTRICITY, SUCH ECCENTRICITY SHALL BE TAKEN INTO ACCOUNT.
- 4.10 FIELD SPLICES SHALL BE POINT OF SPLICE IN BENI IN BENDING, DESIGNED TO DEVELOP THE FULL CAPACITY OF MEMBER AT THE DING, SHEAR AND AXIAL LOAD (COMPRESSION AND TENSION).
- 4.11 PROVIDE STIFFENER PLATES ON EACH SIDE OF WEB OF BEAM OR GIRDER AT POINTS OF CONCENTRATED LOADS. MINIMUM STIFFENER PLATE THICKNESS SHALL BE 1/2" OR FLANGE THICKNESS OF COLUMNS ABOVE OR BELOW, WHICHEVER IS THICKER.
- 4.12 PROVIDE TEMPORARY BRACING OF STRUCTURAL FRAMING TO PROVIDE LATERAL SUPPORT UNTIL ALL PERMANENT BRACING MOMENT CONNECTIONS AND FLOOR AND ROOF DECKS (DIAPHRAGMS) ARE COMPLETELY INSTALLED.

4.13 STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS AND DRAWINGS RELATED TO OTHER TRADES. CONTRACTOR SHALL BE RESPONSIBLE TO CHECK AND COORDINATE DIMENSIONS, CLEARANCES, ETC. WITH THE WORK OF OTHER TRADES. THE STRUCTURAL STEEL CONTRACTOR SHALL PROVIDE FRAMING AROUND OPENINGS IN FLOOR AND ROOF SLAB AS INDICATED IN THE MECHANICAL AND ARCHITECTURAL DRAWINGS.

- 4.15 STRUCTURAL STEEL CONTRACTOR SHALL COORDINATE THE BOTTOM OF BASE WITH THE TOP OF CONCRETE ELEVATION. IN CASE OF CONFLICT, THE CONTRACTOR ALLOWANCE IN HIS BID FOR MORE STRINGENT REQUIREMENTS. PLATE ELEVATION RACTOR SHALL MAKE

5.03

WIND LOAD AS SPECIFIED IN NOTES

COLLATERAL LOAD: 10 PSF

- 4.16 COMPOSITE CONSTRUCTION SHEAR CONNECTORS: SOLID FLUXED SHEAR CONNECTORS STUDS AUTOMATICALLY WELDED THROUGH THE METAL DECK AS SHOWN ON THE DRAWINGS AND IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE MANUFACTURER (NELSON DIVISION OF THE MANUFACTURER) OR APPROVED EQUAL). TRW
- 4.17 ALL STUD WELDING SHALL BE INSPECTED AND FIELD—TESTED. ALL STUDS FAILING THE TEST SHALL BE REPLACED AT THE CONTRACTORS EXPENSE.
- 4.18 PAINT STRUCTURAL STEEL IN ACCORDANCE WITH THE PROJECT SPECIFICATION STEEL SURFACES TO BE ENCASED IN CONCRETE OR RECEIVE SPRAYED ON CONNECTIONS DESIGNATED AS SLIP CRITICAL, OR TO BE WELDED. STEEL SU AUTOMATICALLY WELDED SHEAR CONNECTORS STUDS IN THE FIELD SHALL NO
- 4.20 SEE DRAWINGS FOR STEEL DECK TYPE AND GAUGE. ROOF DECK SHALL BE TYPE B, COATING WIDE RIB.

- FASTEN SIDE LAPS OF ADJACENT UNITS AT A MAXIMUM SPACING OF 24 PUNCHING, OR WITH NO.10 SELF-DRILLING, SELF-TAPPING SCREWS. **INCHES** BY BUTTON

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STRUCTION TINUOUS DRMED BAR ETER

RETE MASONRY UNITS(BLOCK)

-IN-PLACE CONTROL

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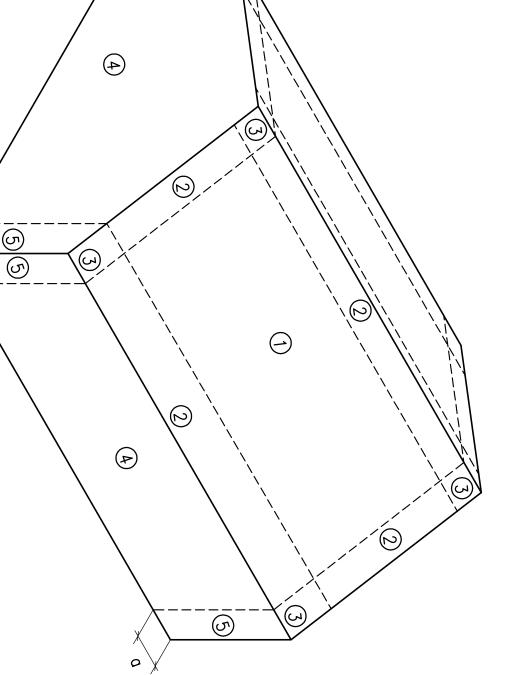
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05-1076 CONSTRUCT WAREHOUSE

SOCS

GENERAL NOTES & WIND LOAD DIAGRAM

TION



OPENING IS PERMITTED FOR EACH BEAM NOT HAVING WEB REINFORCEMENT.	REQUIRED FOR WEB OPENINGS LESS THAN 4" DIAMETER OR 3" X 3". A MAXIMUM OF ONE	SHALL BE LOCATED IN THE MIDDLE 1/3 OF BEAM DEPTH. WEB REINFORCEMENT	STEEL BEAMS AND GIRDERS FOR MECHANICAL AND ELECTRICAL PENETRATIONS. V	THE STRUCTURAL STEEL CONTRACTOR SHALL PROVIDE WEB REINFORCEMENT AT
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5.01

BUILDING SHALL BE CONSTRUCTED IN ACCORDANCE WITH METAL BUILDING MANUFACTURER'S ASSOCIATION (MBMA) CRITERIA.

APP'D

5.00 PRE-ENGINEERED METAL BUILDINGS

5.06

GIRT DEFLECTION CRITERIA:

METAL PANELS: SHEETROCK INTERIOR WALLS: GIRTS SUPPORTING MASONRY:

L/120 L/180 L/420

5.05

DRIFT CRITERIA:

H/100

DOD MINIMUM ANTITERRORISM UNLESS NOTED OTHERWISE.

ACCORDANCE WITH UFC

DESCRIPTION

NS. DO NOT PAINT FIREPROOFING, S RECEIVING PAINTED.

5.07 PROVIDE SUPPORT FOR WALLS BY CONTRACTOR AT EAVES AND DESIGN FRAMES FOR WIND LOAD FROM WALLS BY CONTRACTOR.

)VE FINISH FLOOR)HOR BOLTS

OM OF DECK

SLAB

14NICAL, ELECTRICAL, 1FACTURER 1UM

LEG HORIZONTAL LEG VERTICAL WAY

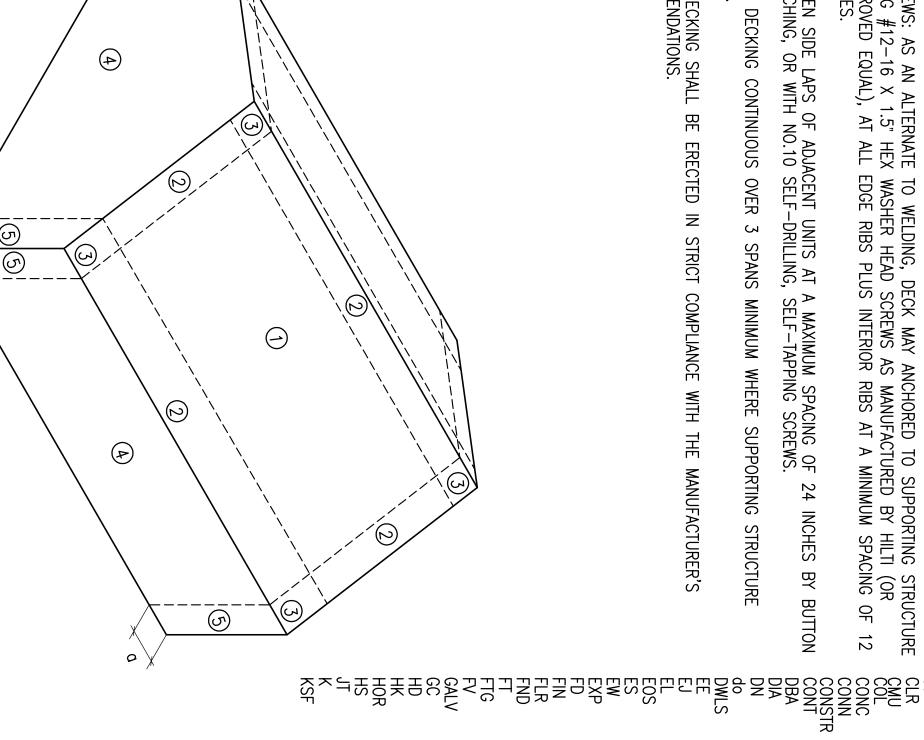
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DATE

- 4.19 FABRICATION AND ERECTION OF STEEL DECKING SHALL CONFORM TO THE LATEST EDITION OF THE STEEL DECK INSTITUTE'S (SDI) "SPECIFICATION AND COMMENTARY FOR COMPOSITE STEEL FLOOR DECK, NON-COMPOSITE STEEL DECK, AND STEEL ROOF DECK" AS APPLICABLE TO THIS PROJECT.
- 4.21 UNLESS NOTED OTHERWISE ALL STEEL DECKING SHALL HAVE A GALVANIZED CONFORMING TO ASTM A525, G60.

4.22 STEEL DECK ANCHORAGE:

- A. SCREWS: AS AN ALTERNATE TO WELDING, DECK MAY ANCHORED TO SUPPORTING STRUCTURE USING #12-16 X 1.5" HEX WASHER HEAD SCREWS AS MANUFACTURED BY HILTI (OR APPROVED EQUAL), AT ALL EDGE RIBS PLUS INTERIOR RIBS AT A MINIMUM SPACING OF 12 INCHES.
- .23 PROVIDE DECKING CONTINUOUS OVER 3 SPANS MINIMUM WHERE SUPPORTING PERMITS. **STRUCTURE**
- .24 STEEL DECKING SHALL BE ERECTED IN STRICT COMPLIANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.



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VERAL CONTRACTOR

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CONTROL JOINT

AIR FORCE SPECIAL

1 SPECIAL OPERATIONS CIVIL ENGINEERING SQUADRON HURLBURT FIELD, FLORIDA

OPERATIONS

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OTHERWISE

(1000 POUNDS) PER SQUARE FOOT

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140	(MPH)	WIND VELOCITY		
С	CATEGORY	EXPOSURE		
22.2	(FT.)	MEAN ROOF HEIGHT	WIND LOAD	
3 on 12	ROOF SLOPE		DETERN	
1.00	FACTOR	IMPORTANCE	MINATION ASSUMPTIONS	
ENCLOSED	CATEGORY	ENCLOSURE		

		DESIG	N WIND F	DESIGN WIND PRESSURES	FOR	OMPONEN ⁻	COMPONENTS AND CLADDING		(PSF)			
EFF. AREA	ROOF ZONE	ZONE 1	-	ROOF ZONE 2		TI	ROOF ZONE 3		WALL ZONE 4	ONE 4	WALL Z	ZONE 5
(SQ. FT)					O-HANG							
< 10	26.2	-41.7	26.2	-87.8	-84.8	26.2	-87.8	-142.7	45.5	-49.3	45.5	-60.9
20	23.9	-40.5	23.9	-79.7	-84.8	23.9	-79.7	-126.4	43.5	-47.3	43.5	-56.8
50	20.8	-39.0	20.8	-69.0	-84.8	20.8	-69.0	-104.9	40.8	-44.6	40.8	-51.3
> 100	18.4	-37.8	18.4	-60.9	-84.8	18.4	-60.9	-88.6	38.7	-42.6	38.7	-47.3
WIND NOTES.												

WIND NOTES:

1. FOR EFFECTIVE AREAS BETWEEN THOSE GIVEN ABOVE

2. THE EDGE STRIP, 'a', = 4.5 FT.

3. PRESSURES SHALL BE APPLIED IN ACCORDANCE WITH THE LOAD MAY 쁌 INTERPOLATED, OTHERWISE JSU 王 LOAD **ASSOCIATED** WTH. 품 LOWER EFFECTIVE AREA

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20 MAR 2009 ESIGNED BY: AWN BY: